

Flood Impact Assessment 180 North Road, Eastwood

Prepared For Mr. George Elias

Project No. **TEL2021185**

City of Ryde

Approved Plans LDA No. LDA2022/0402 Date: 2 March 2023 Council Officer: Colin Murphy Subject to Conditions of Consent

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Disclaimer

The advice and information contained within this report relies on the quality of the records and other data provided by the Client and obtained from Council along with the time and budgetary constraints imposed.

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1 INTRODUCTION

Telford Consulting Pty Ltd have been commissioned to undertake a Flood Impact Assessment for the Proposed Two Storey Dwelling Development at 180 North Road, Eastwood.

This report will:

- 1. Determine the existing stormwater characteristics of the overland flowpath hydraulics and capacity;
- 2. Define the flood risk for the proposed development in accordance with the City of Ryde Council DCP Flood Risk Management Policy;
- 3. Set development levels for the proposed development in accordance with Council's guidelines;
- 4. Discuss risk management in accordance with Council's Flood Risk Management Guidelines; and
- 5. Provide flood risk management procedures for the proposed development.

This report has been prepared generally in accordance with City of Ryde Council advice, Council's Flood Risk Management Policies and other reference documents.

2 SITE DETAILS

2.1 Location

The proposed development site is located within the municipality of City of Ryde Council and is identified as Lot 3 on DP 387071. The property has a total site area of approximately 796.7m² by title. The site is bounded by North Road to the East, by vacant lot to the south and west, and by built up allotments to the north.

Figure 2-1 below shows the site's location outlined in red and the overland flowpath in blue arrows.



Figure 2-1–Site Location

2.2 Proposed Development

The proposed development will see the demolition of the existing structures and the construction of a two storey dwelling development. Access to the site shall be via North Road.

Refer to **Figure 2-2** for the site plan, and **Appendix B** for the architectural plans of the proposed development.



Figure 2-2 – Site Plan

2.3 Topography and Drainage

The topography of the upstream catchment is entirely urbanised area.

Available LIDAR data illustrate the subject site forms part of a local overland flowpath with a catchment area of approximately **<u>2.85ha</u>** in size.

Figure 2-3 below shows the upstream catchment area in blue.



Figure 2-3 - Catchment Map

3 FLOOD IMPACTS

3.1 External Catchment Flow

DRAINS ILSAX model was used for the 1 in 100 year ARI event to analyse the existing catchment and determine the volume of overland flow entering the site.

DRAINS is an integrated hydrological and hydraulic model. It is capable of modelling the hydrology through an ILSAX module including detention storages. Model parameters for sub catchment storages have been selected from recommended design values from the following data sources:

- Catchment roughness values Based on aerial photography, site inspections and previous experience with similar hydrologic assessment; and
- Intensity-Frequency-Duration (IFD) values and rainfall temporal patterns were sourced from the Australian Government, Bureau of Meteorology.

The model result for the overland flowpath's existing urban catchment is shown in the table below:

Table 3-1 – Catchment Flow

Scenario	100year Flow (m³/s)	
2.85ha Existing Overland Flow Catchment	1.61	

Refer to **Appendix C** for DRAINS Model Layout results.

There are several hydraulic parameters often used to simulate overland flows. As seen in **Table 3-2** below, this was not required in this instance.

Table 3-2 - Hydraulic Parameters

Scenario	100 year Peak Flow Capacity (m ³ /s)
Upstream Pipe Network Storage (An allowance for upstream pipe storage has not been made)	N/A
Catchment Bypass (An allowance for bypass such as intercepting roads have not been made as mentioned above)	N/A

3.2 Hydraulic Analysis

The open channel flow hydraulic analysis in HEC-RAS was conducted for the 'pre-development' and 'post-development' scenarios.

A Manning's 'n' value of 0.035 for grass and 0.013 for road pavement areas were adopted.

The existing structures within the neighbouring properties were modelled as full obstruction in the pre-development and post-development scenarios.

The existing structures within the subject site have been also modelled as complete blockage in the pre-development scenario.

For the post development scenario, the proposed development, with exception for the Family, Kitchen and Verandah, was modelled as elevated on piers to allow for overland flow underneath. Refer to **Figure 3-1** for a concept plan.

The proposed driveway into the elevated garage was also modelled as elevated on piers.

As the overland flowpath underneath the elevated footprint might be blocked with time due to vegetation, debris, and other actions, a 70% blockage factor was incorporated for this subfloor area.



Refer to **Figure 3-1** below for a concept plan of the proposed development.

Figure 3-1 - Concept Plan

Figure 3-2 - Hec-Ras sections and Pre Development Flood Extent



Figure 3-3 - Hec-Ras sections and Post Development Flood Extent



Tabulated below in **Table 3-3**, **Table 3-4 & Table 3-5** are the calculated site flood levels, depths and velocities from the external catchment draining onto the proposed development site.

Chainage	100 year	Water Surface	Flow	Velocity	vd
	Peak Flow (m3/s)	Level (m)	Depth (m)	(m/s)	
65	1.610	83.300	0.210	0.990	0.21
60	1.610	82.950	0.180	1.030	0.19
55	1.610	82.640	0.190	1.090	0.21
50	1.610	82.340	0.220	1.150	0.25
45	1.610	82.010	0.220	1.080	0.24
40	1.610	81.670	0.240	1.170	0.28
35	1.610	81.360	0.260	1.130	0.29
30	1.610	81.050	0.260	1.270	0.33
28.5	1.610	80.950	0.240	1.230	0.30
25	1.610	80.750	0.200	1.130	0.23
23.5	1.610	80.740	0.220	0.840	0.18
22.5	1.610	80.730	0.230	0.760	0.17
21	1.610	80.730	0.300	0.600	0.18
20	1.610	80.720	0.250	0.690	0.17
15	1.610	80.610	0.180	1.090	0.20
10	1.610	80.450	0.160	0.860	0.14
5	1.610	80.160	0.140	1.070	0.15
0	1.610	80.160	0.260	0.280	0.07

Table 3-3 - Pre-Development Flood Data

Table 3-4 -	Post-Development	Flood	Data
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Chainage	100 year	Water Surface	Flow	Velocity	vd
_	Peak Flow (m3/s)	Level (m)	Depth (m)	(m/s)	
65	1.610	83.300	0.210	0.990	0.21
60	1.610	82.950	0.180	1.030	0.19
55	1.610	82.640	0.190	1.090	0.21
50	1.610	82.340	0.220	1.150	0.25
45	1.610	82.010	0.220	1.080	0.24
40	1.610	81.670	0.240	1.170	0.28
35	1.610	81.360	0.260	1.200	0.31
30	1.610	81.040	0.250	1.210	0.30
28.5	1.610	80.960	0.240	1.170	0.28
25	1.610	80.790	0.240	1.330	0.32
23.5	1.610	80.750	0.230	1.160	0.27
22.5	1.610	80.730	0.230	1.070	0.25
21	1.610	80.730	0.300	0.750	0.22
20	1.610	80.720	0.250	0.750	0.19
15	1.610	80.610	0.180	1.090	0.20
10	1.610	80.450	0.160	0.860	0.14
5	1.610	80.160	0.140	1.070	0.15
0	1.610	80.160	0.260	0.280	0.07

Chainage	Flow Depth	Pre-Development	Post Development	Difference
	Difference (m)	VD	VD	VD
65	0.000	0.21	0.21	0.00
60	0.000	0.19	0.19	0.00
55	0.000	0.21	0.21	0.00
50	0.000	0.25	0.25	0.00
45	0.000	0.24	0.24	0.00
40	0.000	0.28	0.28	0.00
35	0.000	0.29	0.31	0.02
30	-0.010	0.33	0.30	-0.03
28.5	0.010	0.30	0.28	-0.01
25	0.040	0.23	0.32	0.09
23.5	0.010	0.18	0.27	0.08
22.5	0.000	0.17	0.25	0.07
21	0.000	0.18	0.22	0.04
20	0.000	0.17	0.19	0.02
15	0.000	0.20	0.20	0.00
10	0.000	0.14	0.14	0.00
5	0.000	0.15	0.15	0.00
0	0.000	0.07	0.07	0.00

Table 3-5 - Pre-Development vs	. Post Development Flood Data
--------------------------------	-------------------------------

It is important to note that although there is a slight increase in the VD product and water level at some areas, the VD product and water level are considered acceptable and within council's standards.

As can be seen from **Table 3-5** above, there is an increase in water level up to 0.04 at cross section 25. This increase is limited within the subject site and will not cause any impact on any neighbouring property.

4 CRITERIA FOR SETTING FLOOR LEVELS

All precautions within the planning and design stages of the proposed development should be taken to ensure that the risk of flood impacts are minimised.

City of Ryde Council's DCP sets the development levels for all developments impacted by flooding.

Table 4-1 below shows the minimum floor level for the proposed development in accordance with Council's DCP.

Table 4-1 - Minimum Development Levels

Building Element	Design Requirement	Design Level	
Minimum Habitable Floor Level – Lounge & Media (Interpolating between stations 35 & 40)	1 in 100 year ARI plus 0.50m freeboard	82.02m	
Minimum Habitable Floor Level – Dining & Kitchen (Interpolating between stations 45 & 40)	1 in 100 year ARI plus 0.50m freeboard	82.17m	
Minimum Garage Floor Level (Interpolating between stations 30 & 35)	1 in 100 year ARI plus 0.30m freeboard	81.51m	

5 FLOOD CLASSIFICATION

Three Flood Classifications have been defined as follows:

1. **High Flood Risk Precinct**; This has been defined as the area of land below the 100-year flood event that is either subject to a high hydraulic hazard or where there are significant evacuation difficulties.

The high flood risk precinct is where high flood damages, potential risk to life or evacuation problems would be anticipated, or development would significantly and adversely affect flood behaviour. Most development should be restricted in this precinct. In this precinct, there would be a significant risk of flood damages without compliance with flood related building and planning controls.

 Medium Flood Risk Precinct; This has been defined as land below the 100 year flood event that is not within a High Flood Risk Precinct. This is land that is not subject to a high hydraulic hazard or where there are no significant evacuation difficulties.

In this precinct there would still be a significant risk of flood damage, but these damages can be minimised by the application of appropriate development controls

3. Low Flood Risk Precinct; This has been defined as all land within the floodplain (ie. Within the extent of the probable maximum flood) but not identified within either a High Flood Risk or a Medium Flood Risk Precinct. The Low Flood Risk Precinct is that area above the 100 year flood event.

The Low Flood Risk Precinct is where risk of damages are low for most land uses. The Low Flood Risk Precinct is that area above the 100 year flood and most land uses would be permitted within this precinct.

6 FLOOD SAFETY AND AMENITY ISSUES

The subject site is identified as **Medium Flood Risk.** Council has adopted some restrictions for the following:

- 1. To ensure the proposed development does not result in unreasonable social, economic or environmental impacts upon the amenity or ecology of an area;
- 2. To minimise the safety risk by ensuring the provision of reliable access from areas affected by flooding;
- 3. To minimise the damage to private property and council assets;
- 4. To ensure the proposed development does not have an adverse impact on other properties;
- 5. To ensure all occupants a safe refuge within the site or establish evacuation procedures to a safe refuge above the flood levels outside the site;
- 6. Set all external power points, air conditioning units, hot water systems and pumps for rainwater tanks above the habitable floor levels.

7 FLOOD RESTRICTIONS

<u>Floor Levels</u>: The floor levels of the proposed development are to be in accordance with the advice given in this report (**Section 4**).

Building Components: All proposed structures in the flooded area are to have flood compatible building components below the flood levels. A structural assessment is advised prior to occupation of building by an accredited Structural Engineer.

<u>**Climate Change:**</u> It is recommended to review this flood impact assessment every 10 years particularly with the potential effects of climate change and increased rainfall intensities.

In order for the proposed development to not have adverse impacts on surrounding properties, it is recommended that open style fencing should be adopted within the floodway (if fencing is to be replaced) to ensure no blockages/obstructions to external flows. It is also recommended that no OSD basin retaining walls, garden bed etc. impeded flows within the floodway.

8 FLOOD RISK MANAGEMENT

The proposed development complies with Council's flood evacuation requirements. In order to evacuate, residents would not have to travel through deep water to reach a place of refuge above the PMF flood waters.

It is anticipated that residents could seek refuge above the PMF within the proposed units on the first floor level.

9 CONCLUSION

This Flood Impact Assessment has been prepared to support the Proposed Dual Occupancy Development at 180 North Road, Eastwood.

The report concluded the below:

- The subject site is affected by the 100-year storm event.
- Portion of the proposed development is proposed to be elevated on piers to allow for overland flow underneath.
- Floor levels for the proposed development are addressed in accordance with Council's guidelines and are stated within **Section 4** of the report.

This report shows that the requirements of the Council can be achieved, and therefore recommends that the proposed development proceeds.

10 REFERENCES

- 1. Australia Government, Bureau of Meteorology Website http://www.bom.gov.au/
- 2. City of Ryde Council Development Control Plan 2014
- 3. New South Wales Government Floodplain Development Manual The management of flood liable land, April 2005

11 APPENDICES

Appendix A SURVEY PLAN



Appendix B DEVELOPMENT LAYOUT PLANS





Appendix C DRAINS MODEL LAYOUT & RESULTS

DRAINS Model Layout with results

Sub-catchment name	1 Sul	b-catchment are	ea (ha) 2.85
Hydrological Model Default model You specify	Use C abbreviated data (C more detailed data	Note: The a specify will b calculated fr slope and ro total times o	dditional times you e added to the time om flow path length ughness to get the f concentration.
	Paved Supplem	nentary Gras	sed
Percentage of area	70 0	30)
Additional time (mins)	0 0	0	
Flow path length (m)	250 0	25	0
Flow path slope (%)	7 0	7	
Retardance coefficient n	* 0.012 0	0.0	35
Notes			OK Cancel
Notes		^	Customise Storms

DRAINS DATA

DRAINS results prepared from Version 2021.01							
PIT / NODE DETAILS				Version 8			
Name	Max HGL	Max Pond	Max Surface	Max Pond	Min	Overflow	Constraint
		HGL	Flow Arriving	Volume	Freeboard	(cu.m/s)	
			(cu.m/s)	(cu.m)	(m)		
SUB-CATCHMENT DETAILS							
Name	Max	Paved	Grassed	Paved	Grassed	Supp.	Due to Storm
	Flow Q	Max Q	Max Q	Тс	Тс	Тс	
	(cu.m/s)	(cu.m/s)	(cu.m/s)	(min)	(min)	(min)	
Cat1	1.614	1.337	0.278	3.32	6.31	C	1% AEP, 5 min burst, Storm 1

Appendix D HEC-RAS OUTPUT DATA

HEC-RAS Long Section (Pre-Development Scenario)

HEC-RAS Tabulated Results (Pre-Development Scenario)

HEC-RAS Plan: Plan 01 River: 1 Reach: 0 Profile: PF#1										1		
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
0	65.00	PF#1	1.61	83.09	83.30	83.30	83.35	0.018845	0.99	1.63	12.81	0.89
0	60.00	PF#1	1.61	82.77	82.95	82.95	83.01	0.025032	1.03	1.56	14.27	1.00
0	55.00	PF#1	1.61	82.45	82.64	82.64	82.70	0.024048	1.09	1.48	12.10	1.00
0	50.00	PF#1	1.61	82.12	82.34	82.34	82.41	0.023266	1.15	1.40	10.29	1.00
0	45.00	PF#1	1.61	81.79	82.01	82.01	82.07	0.025231	1.08	1.49	12.85	1.01
0	40.00	PF#1	1.61	81.43	81.67	81.67	81.74	0.023389	1.17	1.38	10.02	1.00
0	35.00	PF#1	1.61	81.10	81.36	81.36	81.43	0.023483	1.13	1.42	10.73	1.00
0	30.00	PF#1	1.61	80.79	81.05	81.05	81.13	0.023295	1.27	1.27	7.70	1.00
0	28.50	PF#1	1.61	80.71	80.95	80.95	81.02	0.023146	1.23	1.31	8.39	1.00
0	25.00	PF#1	1.61	80.55	80.75	80.75	80.82	0.023481	1.13	1.42	10.76	1.00
0	23.50	PF#1	1.61	80.52	80.74		80.77	0.010753	0.84	1.93	12.83	0.69
0	22.50	PF#1	1.61	80.50	80.73		80.76	0.008625	0.76	2.12	13.87	0.62
0	21.00	PF#1	1.61	80.43	80.73		80.75	0.004498	0.60	2.67	15.12	0.46
0	20.00	PF#1	1.61	80.47	80.72		80.74	0.007277	0.69	2.34	15.64	0.57
0	15.00	PF#1	1.61	80.43	80.61	80.61	80.68	0.025609	1.09	1.47	12.61	1.02
0	10.00	PF#1	1.61	80.29	80.45	80.45	80.49	0.020196	0.86	1.87	19.03	0.88
0	5.00	PF#1	1.61	80.02	80.16	80.16	80.22	0.031966	1.07	1.51	15.77	1.10
0	0.00	PF#1	1.61	79.90	80.16	80.03	80.17	0.001000	0.28	5.65	31.86	0.22

HEC-RAS Long Section (Post-Development Scenario)

HEC-RAS Tabulated Results (Post-Development Scenario)

HEC-RAS Plan: Plan 01 River: 1 Reach: 0 Profile: PF#1										1		
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(m3/s)	(m)	(m)	(m)	(m)	(m/m)	(m/s)	(m2)	(m)	
0	65.00	PF#1	1.61	83.09	83.30	83.30	83.35	0.018845	0.99	1.63	12.81	0.89
0	60.00	PF#1	1.61	82.77	82.95	82.95	83.01	0.025032	1.03	1.56	14.27	1.00
0	55.00	PF#1	1.61	82.45	82.64	82.64	82.70	0.024048	1.09	1.48	12.10	1.00
0	50.00	PF#1	1.61	82.12	82.34	82.34	82.41	0.023266	1.15	1.40	10.29	1.00
0	45.00	PF#1	1.61	81.79	82.01	82.01	82.07	0.025231	1.08	1.49	12.85	1.01
0	40.00	PF#1	1.61	81.43	81.67	81.67	81.74	0.023389	1.17	1.38	10.02	1.00
0	35.00	PF#1	1.61	81.10	81.36	81.36	81.43	0.025964	1.20	1.35	10.00	1.04
0	30.00	PF#1	1.61	80.79	81.04	81.04	81.12	0.028230	1.21	1.33	8.87	1.00
0	28.50	PF#1	1.61	80.72	80.96	80.96	81.03	0.030068	1.17	1.37	9.66	0.99
0	25.00	PF#1	1.61	80.55	80.79	80.79	80.88	0.031257	1.33	1.21	6.63	1.00
0	23.50	PF#1	1.61	80.52	80.75	80.74	80.82	0.025407	1.16	1.39	8.31	0.90
0	22.50	PF#1	1.61	80.50	80.73	80.72	80.79	0.023889	1.07	1.50	10.09	0.89
0	21.00	PF#1	1.61	80.43	80.73		80.76	0.009491	0.75	2.14	13.12	0.59
0	20.00	PF#1	1.61	80.47	80.72		80.75	0.009673	0.75	2.16	14.52	0.62
0	15.00	PF#1	1.61	80.43	80.61	80.61	80.68	0.025609	1.09	1.47	12.61	1.02
0	10.00	PF#1	1.61	80.29	80.45	80.45	80.49	0.020196	0.86	1.87	19.03	0.88
0	5.00	PF#1	1.61	80.02	80.16	80.16	80.22	0.031966	1.07	1.51	15.77	1.10
0	0.00	PF#1	1.61	79.90	80.16	80.03	80.17	0.001000	0.28	5.65	31.86	0.22

